

CHAPTER 76

CALCULATED SAND FILLS AND GROIN SYSTEMS

by

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Abstract

This paper demonstrates the results of the comparison between the calculated alignment of the beach's plan view and the actual alignment that resulted from the natural wave action.

A new groin system was designed to replace an existing one. The new groin system and spacing was designed to hold the beach fill at the required width during average wave conditions as well as changing storm wave conditions. The system was designed for Las Tunas Beach, a beach near Los Angeles, California.

In other words, the energies are tabulated for all storm conditions, and the beach alignment is recalculated for changing storm conditions. As the beach plan view alignment rotates back and forth with changing storm directions, it must always meet the minimum beach width requirements. This may entail changing the groin lengths, heights and spacing several times before the design complied with all requirements.

Surveys from 1929 to 1970 showed that the beach did not have long periods, periods of several years, of constant erosion or constant accretion. If there are no major man made changes, it is safe to assume that nature will provide an adequate sand supply.

Last, but not least, the calculated beach alignments check with the empirical information gathered from years of surveying.

This paper tells of a groin system and sand fill that was calculated by the use of resultant wave energies.

The beach to be studied was widened during 1929 by damming up the littoral drift with five sheet pile groins. We plan to replace the five groins with two long rock groins and fill the groin system with sand. The new groin system and beach alignment was calculated and compared to existing alignments that have been surveyed over the years.

Plate 1 shows the beach as it was seen recently and the way it will look after widening. This beach, called Las Tunas Beach, is located along the Pacific Ocean shoreline, a few miles westerly of the City of Santa Monica in the County of Los Angeles, California. This is, of course, within the United States.

Because of the amount of survey data available over a great number of years, the natural beach has been examined much like it would be if it were a laboratory model. Surveys have been taken at various seasons of the year and during changes of wave climates.

Since the beach's five long groins dammed up the littoral drift, surveys of the natural beach would tell us how the littoral drift was impounded and indicate the plan view alignment changes with different wave climates.

Plate 2 shows the locations of our range lines along which surveys of the ocean bottom were repeated and compared.

See Plate 1 for the location of five groins and the remaining sand fillet.

Our range lines are spaced 250 feet apart along this 1500 feet of proposed shoreline development.

Plate 3 demonstrates our method of plotting and comparing the profiles.

The beach foreshore slopes remain somewhat parallel during seasonal and long term changes.

The groins have deteriorated years ago and allowed the beach to return to its present position. Since the existing beach has remained in this position, except for seasonal changes, since surveyed during the early 1950's, it is evident that there is adequate nourishment and the beach is stable.

Plate 4 shows the comparison between the calculated alignment and present alignment. The calculated alignment was made for storms coming from south, southeast to west. The shore line on this plate goes east and west and you will note the difference in the resultant inshore wave directions.

The predominant inshore wave direction changes only slightly with different storms, while seaward of the islands the storms varied more than 120° in direction.

A more detailed discussion of what wave statistics are used will be covered later. At this time let's discuss the different alignments. Note the alignments from the westerly

PLATE 1

LAS TUNAS BEACH IMPROVEMENT PROJECT

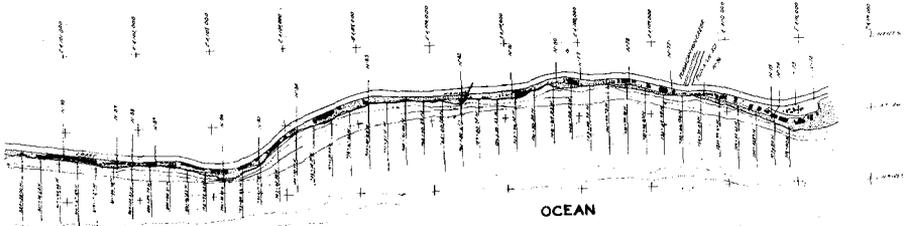


BEFORE BEACH IMPROVEMENT



AFTER BEACH IMPROVEMENT

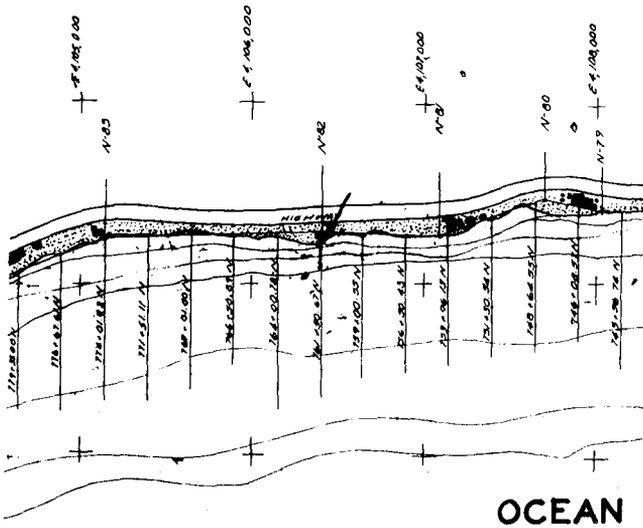
PLATE 2



NUMBER STATION	ADJUSTED STATION	GRID COORDINATES NORTH EAST	FROM	TO	GRID BEARING	DISTANCE
N 81	754187.16	4128.7411 100085.81	N 80	754187.16	0°00'00"	0.00
N 82	761167.88	4128.76283 100026.16	N 81	754187.16	113°00'00"	113.27
N 83	772115.95	4128.78264 100137.84	N 82	761167.88	113°00'00"	113.27
N 84	782164.53	4128.80285 100148.53	N 83	772115.95	113°00'00"	113.27
N 85	792164.07	4128.82286 100094.07	N 84	782164.53	113°00'00"	113.27
N 86	796139.45	4128.84287 100074.45	N 85	792164.07	113°00'00"	113.27
N 87	805170.14	4128.86288 100025.14	N 86	796139.45	113°00'00"	113.27
N 88	809482.58	4128.88289 100025.58	N 87	805170.14	113°00'00"	113.27
N 89	811287.77	4128.90290 100026.77	N 88	809482.58	113°00'00"	113.27
N 90	814185.01	4128.92291 100027.01	N 89	811287.77	113°00'00"	113.27

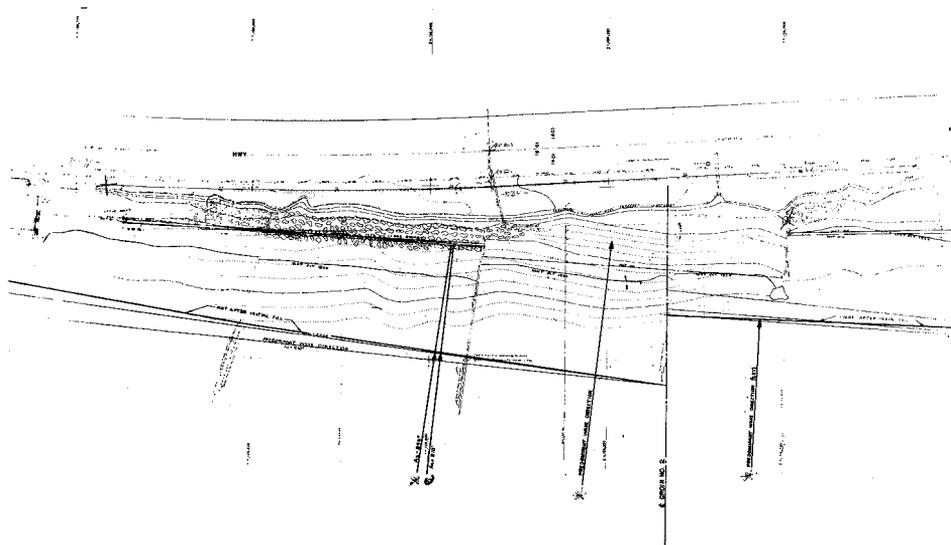
NUMBER STATION	FIELD BOOK STATION	GRID COORDINATES NORTH EAST	FROM	TO	GRID BEARING	DISTANCE
N 77	711164.5	4128.66100 100172.23	N 76	702115.95	113°00'00"	113.27
N 78	721164.5	4128.68100 100223.50	N 77	711164.5	113°00'00"	113.27
N 79	731164.5	4128.70100 100274.77	N 78	721164.5	113°00'00"	113.27
N 80	741164.5	4128.72100 100326.04	N 79	731164.5	113°00'00"	113.27
N 81	751164.5	4128.74100 100377.31	N 80	741164.5	113°00'00"	113.27
N 82	761164.5	4128.76100 100428.58	N 81	751164.5	113°00'00"	113.27
N 83	771164.5	4128.78100 100479.85	N 82	761164.5	113°00'00"	113.27
N 84	781164.5	4128.80100 100531.12	N 83	771164.5	113°00'00"	113.27
N 85	791164.5	4128.82100 100582.39	N 84	781164.5	113°00'00"	113.27
N 86	801164.5	4128.84100 100633.66	N 85	791164.5	113°00'00"	113.27
N 87	811164.5	4128.86100 100684.93	N 86	801164.5	113°00'00"	113.27
N 88	821164.5	4128.88100 100736.20	N 87	811164.5	113°00'00"	113.27
N 89	831164.5	4128.90100 100787.47	N 88	821164.5	113°00'00"	113.27
N 90	841164.5	4128.92100 100838.74	N 89	831164.5	113°00'00"	113.27

Enlarged Section



This Plate shows the onshore beach control line with "N" monuments and lines extending seaward that formed the paths for the beach profile. Direction and scale can be determined from available survey data.

PLATE 4A



- * Azimuth 246° is a deep water wave from the west.
- Azimuth 210° is a deep water wave from the south.
- * Azimuth 232° is the predominant wave direction.

PLATE 4B

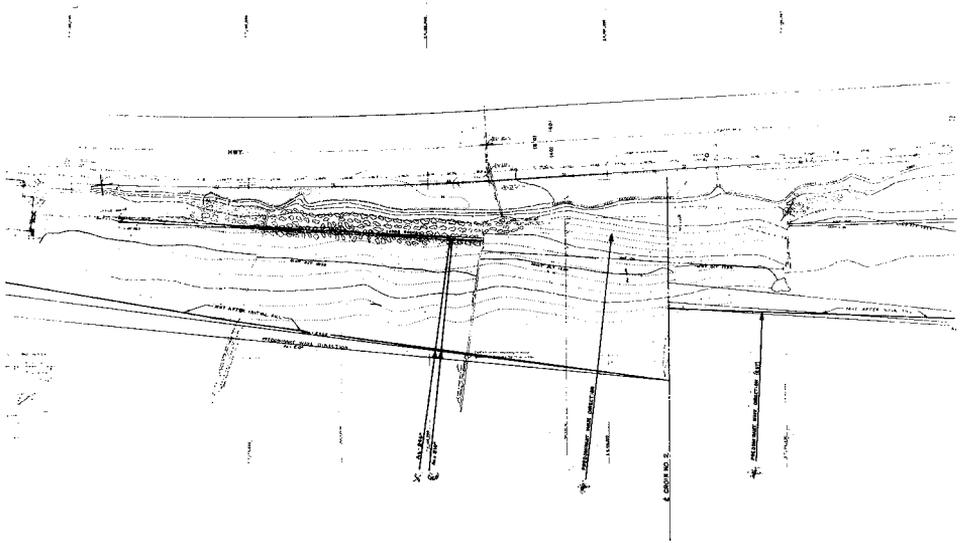
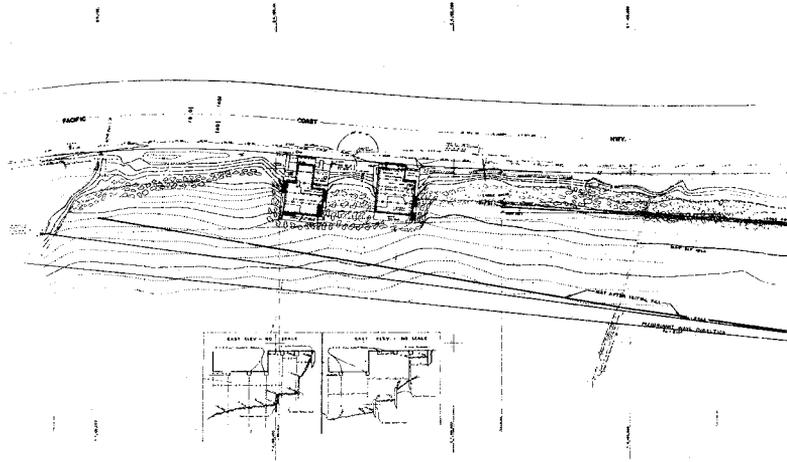


PLATE 4C



waves as they compare to those from the south and the predominant wave direction of all waves.

Since the sand fillet will attempt to align itself normal to the wave orthogonals from these predominant wave directions, we can calculate the alignment of the shoreline by knowing the inshore direction of the orthogonals.

The next question is how did we calculate these different shoreline alignments and where did we get the wave statistics to do it with.

Plate 5 shows the crux of the problem. The hindcasted wave statistics must be brought through the island maze, refraction and resultant wave directions must be calculated.

Note where the hindcasted wave statistics were compiled, Station A and Station 7, and where the wave statistics were tabulated inside the island maze, *lat. 33° 56', long. 118° 37'*.

Plate 6 shows a summary of the work, and an explanation of the various steps that were followed.

Column No. 1 shows the wave directions at Station A, Station 7 and those used to tabulate the local winds.

The waves at Stations A and 7 were compiled within 22½ degrees arcs, of which the directions shown are the center. The local waves or those called sea, are recorded within ten degree arcs, and the directions shown are the center of these arcs.

Column No. 2 shows the resultant wave directions, based on energy calculation, of those waves shown in Column No. 1.

The first calculations were to change the wave statistics to deep water wave energy. Since relative energy was all that was needed, we calculated the total storm wave energy by the use of significant wave heights. You will note, I used the word "total". In other words, we calculated the wave energy for the total average annual time that given storms would occur during the year.

The next step was to calculate the resultant energy for all the storms within the 22½ degree segments represented in Column No. 1.

The resultant can be calculated by vector analysis. The length of the vector represents total annual energy for storms and the vector direction represents the direction of these storm waves. By joining these vectors end for end the resultant energy and direction can be calculated.

Column No. 3 shows a tabulation of the shift in the direction of the resultant of wave energy when the point of observation is moved from Station 7 or A to the deep water area just off Las Tunas Beach. This shift will be explained in a later plate.

Column No. 4 is a summation of Columns 2 and 3.

Column No. 5 shows the resultant relative energy that has been calculated for different storm conditions at Station A or Station 7.

Column No. 6 shows a coefficient that indicates the decimal part of this energy that gets through the island maze.

CALCULATED SAND FILLS

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PLATE 6
 WAVE STATISTICS
 AT
 NORTH LATITUDE 33° 55.9'
 AND
 WEST LONGITUDE 113° 36.7'

(ANNUAL ENERGY IN MILLIONS OF FOOT POUNDS)

(1) DIRECTION (AZIMUTH)	(2) RESULTANT DIRECTION (AZIMUTH NORTH)	(3) SHELTERING CORRECTION TO DIRECTION	(4) DIRECTION AT THE ABOVE LOCATION	(5) ENERGY AT STATION A or Z	(6) ISLAND SHELTERING COEFFICIENT (C) C } C ²		(7) RESULTANT ENERGY AT THE ABOVE LOCATION
			SWELL STATION A				
157.5	157.3°	+29.9	187.2°	10,400	.33	.109	1,140
180.0	174.8°	+21.9	196.7°	8,400	.47	.221	1,850
202.5	204.6°	+ 6.0	210.6°	9,500	.60	.360	3,420
225.0	226.0°	+ 3.5	229.5°	29,200	.71	.504	14,900
			STATION 7				
247.5°	247.5°	- 8.5	239.0°	5,600	.62	.384	2,130
270.0°	270.0°	-24.0	246.0°	48,500	.54	.292	14,050
			SEA				
120	145	Waves are the result of local	145				22
130	148	winds blow- ing towards	148				7
140	157	the main- land from	157				53
150	167	the islands --no shel- tering	167				84
160	175	correction needed.	175				46
170	182		182				78
180	187		187				50
190	194		194				137
200	204		204				217
210	213		213				190
220	217		217				65
230	219		219				180
240	226		226				120

RESULTANT - WAVE DIRECTION
 (For all Waves listed) = 231° 50'
 AVERAGE WAVE PERIOD = 11.8"

NOTE: Waves from west north west have no effect on this area.

We call this the island sheltering coefficient.

Column No. 7 shows the fractional part of the energy that reaches the deep water area just off Las Tunas Beach.

Three important beach forming wave resultants exist, wave resultants from westerly waves, southwesterly waves and southerly waves.

Other waves considered in the tabulation are as follows:

The waves from the southwest have a very high energy value but are so close to the predominant wave direction that no special consideration need be given them.

The local wind wave energies are tabulated at the bottom of the plate under the term sea. None of these waves have significant quantities of energy in the study area. Furthermore, the waves from 226° to 145° occur over a wide range of months and involve many small storms during the year. Thus, the energy for any individual storm will be quite small and will not be very effective in shaping the study area coastline.

Plate 7 shows the way the island blocking is handled. The details of this type of blocking can not be completely covered here, but in general the area under the curve shown represents total energy from the average storms and the area under the curve and not crosshatched shows the part of the energy that gets through the islands.

Because of the irregularities of wind waves within the island maze, it was expedient to divide the problem into two parts --energy resulting from deep water swells, and energy resulting from fetch, wind and local conditions.

Since little wave decay exists between Station A or 7 and Las Tunas Beach, the plate shows how the island sheltering affected a wave with an azimuth of 175° from Station A.

In the formula $ide = K(1 + \cos 2\theta)de$, θ is the angle on either side of the center of the waves formed and ide is the energy for a small sector of θ . The K and de can be chosen to be unity without significant error. Because this wave is a deep water swell, a path of 45° on either side of 175° is considered and the blocking effect is shown. The energy will be reduced by 22% of the energy at Station A and the resulting wave azimuth will increase from 175° to 197°--this direction change is achieved by calculating the resultant of the energy in the open windows.

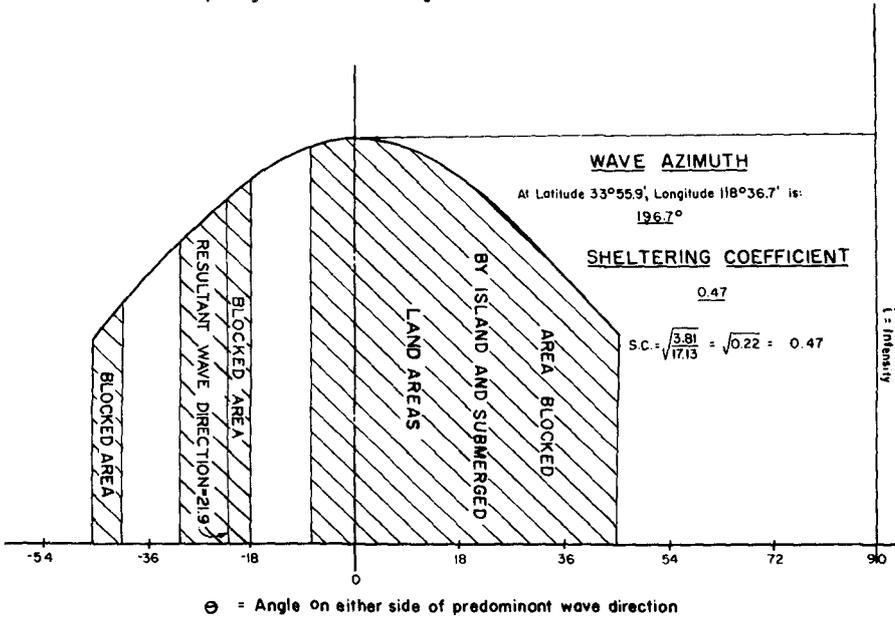
The unblocked energy is represented by the figure 17.13 which is the area under the total curve, and 3.81 represents the area of the open windows. Therefore, the energy just off Las Tunas Beach equals energy calculated at Station A, times $(3.81 \div 17.13)$ and equals 22% of the total energy.

During sea conditions in which the sheltering land masses are within the fetch area, a wave path of 90° on either side of the fetch directions is considered to be good. Decayed swell conditions can lead to overestimating the effects of island sheltering. The variability of wave direction for swells can be less than 45° and may be only a few degrees for very distant storms. Remember, the island windows are based on the average

PLATE 7

WAVE DIRECTION 174.8° AT STATION "A"

Chart for Computing Island Sheltering Coefficient $K = K(1 + \cos 2\theta)d\theta$



Predominant wave direction = _____

wave period for all swells, and the long period swells will not get through windows as large as those shown. Forty-five degrees probably represents an average figure. Beaches in Los Angeles County are developed, primarily by the mid-range wave conditions and not the unusual rare occurring large wave conditions.

Investigating island sheltering necessitates considering the depth to which this island hocking will take place.

This was accomplished by determining the average wave period and taking 50 percent of the resultant deep water wave lengths as the depth to which the offshore islands will block continental hound waves.

The refraction diagrams were calculated and used to determine the resultant inshore energies.

Plate 8 shows the plotted profiles and the grouted groin area. The grout is placed on the porous cap stone to make it impermeable far enough seaward to maintain the beach at a minimum width.

The last ingredient is the question of an adequate, continuous sand supply. Surveying the beach over a great many years also gives us the answer to this question. Surveys from 1929 to 1970 showed that the beach did not have long periods--periods of several years of constant erosion and constant accretion. If there are no major man made changes, it is safe to assume that nature will provide an adequate sand supply.

Last, but not least, the calculated beach alignments check with the empirical information gathered from years of surveying.

REFERENCES

1. Shore Protection Planning and Design
(Technical Report No. 4, Third Edition)
Department of the Army, Corps of Engineers,
published 1966
2. Wave Statistics for Seven Deep Water Stations
Along the California Coast
National Marine Consultants, 1960
3. A Statistical Survey of Ocean Wave Characteristics
In Southern California Waters
Marine Advisers, January, 1961
4. A Lognormal Size Distribution Model For Estimating
Stability of Beach Fill Material
Technical Memorandum No. 16, Department of the
Army, Corps of Engineers Research Center.
5. Beach Improvement and Erosion Control Report
Design Division, Department of County Engineer,
August, 1965
6. Santa Monica Canyon to Las Flores Shoreline
Survey
County Engineer Field Books